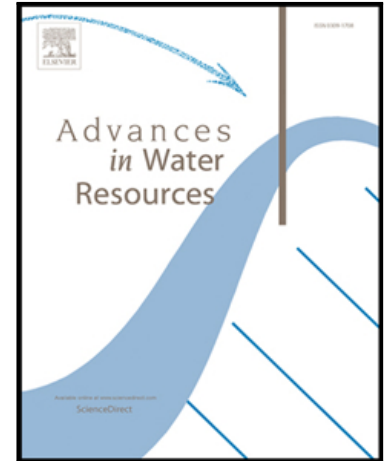


Accepted Manuscript

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PII: S0309-1708(16)30015-X
DOI: [10.1016/j.advwatres.2016.02.003](https://doi.org/10.1016/j.advwatres.2016.02.003)
Reference: ADWR 2553



To appear in: *Advances in Water Resources*

Received date: 22 October 2015
Revised date: 1 February 2016
Accepted date: 4 February 2016

Please cite this article as: S. Grimaldi, A. Petroselli, G. Salvadori, C. De Michele, Catchment compatibility via copulas: a non-parametric study of the dependence structures of hydrological responses, *Advances in Water Resources* (2016), doi: [10.1016/j.advwatres.2016.02.003](https://doi.org/10.1016/j.advwatres.2016.02.003)

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Highlights

- dependence structures as signature of hydrological responses
- compatible catchments have similar dependence structures
- it is possible to merge observed data for improving copula estimation in compatible catchments

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Catchment compatibility via copulas: a non-parametric study of the dependence structures of hydrological responses

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Abstract

The similarity of catchment responses is a fundamental issue for regionalization studies, and hydrograph attributes (i.e., Discharge Peak, Volume, and Duration) can reveal the signature and the synthesis of local scale processes. Here, we focus the attention on the “compatibility” between catchments, viz. on the possibility

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¹The support of the CRM-CANSSI (Université de Montréal, Montréal (Québec), Canada), where the work originated, is gratefully acknowledged.

²Helpful discussions with C. Sempi (Università del Salento, Italy), F. Durante (Free University of Bozen-Bolzano, Italy), I. Kojadinovic (Université de Pau et des Pays de l'Adour, France), and C. Genest (McGill University, Montréal (Québec), Canada) are acknowledged. The support of the CRM-CANSSI (Université de Montréal, Montréal (Québec), Canada), where the work originated, is gratefully acknowledged. The support of the CMCC (Centro Euro-Mediterraneo sui Cambiamenti Climatici, Lecce (Italy)) is acknowledged.

to transfer, from one catchment to another, the information about the dependence structures at play. In particular, we statistically investigate the possible relationships between the features of different Basin Scenarios (characterized via the Concentration Time T_c and the Curve Number CN) and the corresponding dependence structures ruling the joint statistics of Discharge, Volume, and Duration. Given a large set of synthetic runoff time series, generated via a rainfall-runoff model, recent non-parametric tests, based on empirical copulas, are used to compare the dependence structures associated with different soil uses and concentration times. The results indicate how the hydrological properties may affect the dependence structure. The outcomes of the investigation could be particularly effective in two practical applications: (1) for determining the degree of compatibility of the dependence structures associated with different basin scenarios, and (2) for enriching scanty data bases, in order to improve the estimation of multivariate copulas.

Keywords: Copula, Catchment compatibility, Rainfall-runoff transformation, Non-parametric test

1. Introduction

The concept of “hydrologic similarity”, or “catchment similarity”, stems from the idea that, despite the complexity of catchment response due to heterogeneities and different mechanisms involved, it exists a certain level of organization and

5 predictability of the catchment behavior. This concept allows to transfer the in-
6 formation from gauged to ungauged basins, and infer the hydrologic behavior in
7 basins where no measurements are available (a common situation in hydrological
8 practice).

9 The identification of hydrologic similarity requires a catchment classification
10 (McDonnell and Woods, 2004), and consequently a catchment pooling, i.e. group-
11 ing basins which are considered hydrologically similar: this operation is not an
12 easy task (Ali et al., 2012). In some cases, it results in a geographic region (also
13 known as homogeneous region), in others in a cluster of basins (also known as
14 region of influence), which has not necessarily the spatial contiguity, and may
15 raise problems of spatial extrapolation when a leopard skin spatial distribution is
16 obtained (Merz and Blöschl, 2005).

17 In literature, the hydrologic similarity has been considered to gain insights
18 into the processes of runoff generation (see, e.g., Beven et al. (1988); Larsen et al.
19 (1994); Blöschl and Sivapalan (1995); Robinson and Sivapalan (1997)), and to
20 group together catchments for regional flood frequency analysis, and low-flow
21 regionalization techniques (Laaha and Blöschl, 2006a).

22 In the last decades, several techniques have been developed to provide a catch-
23 ment classification and investigate the hydrologic similarity. Some of these are

24 based on statistical simple or multiple scaling (Gupta and Waymire, 1990; Gupta
25 and Dawdy, 1995; Gupta et al., 1994; Robinson and Sivapalan, 1997; De Michele
26 and Rosso, 2002). Others have used the seasonality of discharges, through indices
27 like Pardé or Burn ones (Pardé, 1947; Burn, 1990, 1997; Merz et al., 1999), or
28 introducing a seasonality histogram (Laaha and Blöschl, 2006b), or metrics based
29 on the mean seasonal water balance (Coopersmith et al., 2012; Ye et al., 2012; Pe-
30 tersen et al., 2012; Berghuijs et al., 2014). Other works have adopted cluster anal-
31 ysis techniques, like agglomerative hierarchical clustering, k-means clustering,
32 and fuzzy partition clustering (Sawicz et al., 2011). Lastly, other approaches have
33 adopted a combination of some of the above mentioned criteria (De Michele and
34 Rosso, 2002). Despite the huge efforts devoted to the investigations of hydrologic
35 similarity and catchment classification, and the results obtained, the uncertainty
36 associated with the results requires further analyses.

37 The dependence structure among hydrological variables has recently become
38 an intensively investigated topic for its pivotal implications in practical applica-
39 tions (Salvadori et al., 2007). The increasing interest, confirmed by a growing
40 number of papers published on this issue (see www.stahy.org for a list of papers
41 produced in the last ten years), is mainly due to the introduction of copulas and
42 related tools (Sklar, 1959; De Michele and Salvadori, 2003). Indeed, the opportu-

43 nities offered by copulas have attracted many researchers on multivariate analysis
44 that, nowadays, are transposing several traditional univariate hydrological studies
45 into a multivariate perspective. In particular, according to the statement reported
46 in the Directive 2007/60/EC of The European Parliament and of The Council (The
47 European Parliament and The Council (6.11.2007)), a multivariate approach in-
48 volving a number of non-independent variables should be considered for assessing
49 the threatening of the phenomenon under investigation (The European Parliament
50 and The Council, 6.11.2007, p. 31, Chapter III, Article 6.4).

51 In the last decade, several authors focused their attention on multivariate fre-
52 quency analysis of hydrological variables, with the aim either to characterize their
53 dependence structures (Salvadori and De Michele, 2004; Favre et al., 2004; Re-
54 nard and Lang, 2007; Shiau et al., 2007; De Michele et al., 2007; Salvadori and De
55 Michele, 2010, 2011; Haslauer et al., 2012; Salvadori et al., 2013; Louie, 2014),
56 or to provide new insights for estimating multivariate design quantiles (Salvadori
57 et al., 2011; De Michele et al., 2013; Gräler et al., 2013; Volpi and Fiori, 2014;
58 Salvadori et al., 2015; Salvadori and De Michele, 2015). Moreover, a significant
59 effort was devoted to include multivariate distributions in theoretical schemes for
60 improving the performance of hydrological models (De Michele and Salvadori,
61 2003; Salvadori and De Michele, 2006; De Michele et al., 2007; Gao et al., 2007;

62 Bárdossy and Pegram, 2013; Motamedi and Liang, 2014; Hao and Aghakouchak,
63 2014).

64 Recently, the interest is spanning on new frontiers assuming that the depen-
65 dence structure intrinsically contains a paramount of information about the dy-
66 namics of hydrological processes. For instance, multivariate trend analyses were
67 applied (Ben Aissia et al., 2014; Huang et al., 2014; Chebana et al., 2013; Bender
68 et al., 2014) assuming that the dependence structure could vary in time (and/or in
69 space), enhancing the interpretation of its variability with respect to the classical
70 univariate trend analysis. Interesting studies were also proposed concerning the
71 use of copulas for evaluating the performance of hydrological models by compar-
72 ing the dependence structure of simulated variables (Vandenberghe et al., 2011;
73 Pham et al., 2013).

74 Particularly attractive is also the regional analysis of flood properties offered
75 by Gaál et al. (2015), where the idea to relate the dependence structure of flood
76 Discharge and Volume to the watershed physical properties is proposed. These
77 authors illustrate an application to 330 catchments in Austria, characterized by
78 different dimensions, climatic forcings, altitude, and presence of snow: although
79 affected by relevant uncertainty due to the sample dimension and the strong het-
80 erogeneity of the hydrological and physical conditions, the results are encourag-

81 ing. Samaniego et al. (2010) proposed a measure of dissimilarity between basins
82 based on the pairwise comparison of the empirical copula densities of runoff, and
83 applied this to 38 German basins. Further contributions are available, with the
84 aim to generalize the univariate regional procedure to a multivariate context (see,
85 e.g., Salvadori and De Michele (2010); De Michele et al. (2013); Salvadori and
86 De Michele (2015)).

87 The concept of “hydrologic similarity” is a notion featuring many facets, that
88 leaves the door open to several possible definitions: in fact, the literature reports
89 a number of approaches, based on different indices and techniques (Ali et al.,
90 2012). This is probably due to the complexity of the problem, and to the fact
91 that different indices account for distinct aspects of hydrologic similarity. Here,
92 we focus the attention on a specific, yet unexplored, facet of the problem we
93 name “catchment compatibility”, which refers to the possibility of transferring,
94 from one catchment to another, the information about the dependence structures
95 at play, i.e. those ruling the joint behavior of the relevant variables. In particular,
96 we statistically investigate the possible relationships between the features of Basin
97 Scenarios (characterized by the Concentration Time T_c and the Curve Number
98 CN) and the ones of the corresponding dependence structures regulating the joint
99 statistics of Discharge, Volume, and Duration.

100 Long synthetic runoff time series are generated via a rainfall-runoff model,
101 in order to make available large samples of hydrograph attributes related to spe-
102 cific basin scenarios, for different soil uses and concentration times. Recent non-
103 parametric tests, based on empirical copulas, are used to compare the dependence
104 structures associated with each basin scenario. Moreover, several strategies of
105 comparison, either individual or ensemble, are proposed adopting statistical dis-
106 tances and equality tests between copulas. Among others, practical benefits con-
107 sist in a valuable support to decide whether, and to what extent, time series col-
108 lected in a basin characterized by a given pair (T_c, CN) could be used in order to
109 improve the (statistical) knowledge of the copulas at play in another (partially or
110 totally) ungauged basin with different T_c - CN features. Actually, since a crucial
111 drawback in copula applications is often represented by the size of the available
112 samples, we introduce, and test, a procedure to merge multivariate data sets col-
113 lected in “compatible” basins, in order to improve the copula inference.

114 2. Materials

115 In this Section, details concerning the catchment case study, the framework
116 for generating runoff time series, and the simulation experiment are provided.

117 2.1. *The case study*

118 The selected catchment is the Torbido river basin, located in central Italy. Tor-
119 bido river is a small tributary of the Tiber, and is characterized by a drainage area
120 of 61.67 km², elevations ranging from 85 to 625 m a.s.l., an average watershed
121 slope of 21.9%, and a maximum distance between divide and outlet of 25.81 km.
122 The Italian Geographic Military Institute provided the DEM with integer preci-
123 sion and a 20-m resolution (Italian Geographic Military Institute (IGMI), 2003),
124 while land cover was retrieved from the website CORINE (European Environ-
125 mental Agency (EEA), Programme of the European Commission, 2000): Table 1
126 summarizes the main features of the Torbido catchment.

127 Observed rainfall data are available from the Viterbo rain gauge station, lo-
128 cated 30 km from the catchment centroid, for a period of 61 years (from 1951 to
129 1983, and from 1987 to 2014) at a daily time scale, and for a period of 10 years
130 (from 2005 to 2014) at a 1-hour resolution.

131 2.2. *The hydrologic model*

132 The adopted hydrological model is the recently developed simulation frame-
133 work named COSMO4SUB (COntinuous Simulation MOdel For Small Ungauged
134 Basins — see Grimaldi et al. (2012b,c)). COSMO4SUB consists in a multi-
135 step procedure characterized by a rainfall generator, an infiltration model, and

136 a rainfall-runoff model.

137 Regarding the first step, the HyetosR 1.1 software was used (see Koutsoyian-
138 nis and Onof (2001), and www.itia.ntua.gr/en/softinfo/3/). HyetosR is
139 a package for the temporal stochastic simulation of rainfall process at fine time
140 scales (say, less than one day) that uses (1) the Bartlett-Lewis rectangular pulses
141 rainfall model (Onof and Wheater, 1993, 1994), and (2) suitable disaggregation
142 techniques (Koutsoyiannis, 2001) that adjust the finer scale values in order to ob-
143 tain the required coarser scale values without affecting the stochastic structure of
144 the data. The model was calibrated on the observed rainfall data.

145 Regarding the second step, the Soil Conservation Service-Curve Number (SCS-
146 CN) method (USDA-SCS, 1986) was used in order to calculate the rainfall ex-
147 cess. The SCS-CN method was applied assigning the value 0.2 to the parameter
148 involved in the calculation of initial abstraction, as suggested in the official for-
149 mulation (USDA-NRCS, 2010).

150 Regarding the third step, a geomorphological rainfall-runoff model was ap-
151 plied to the excess rainfall time series estimated in the previous step, in order to
152 obtain the corresponding runoff time series. An advanced version of the well-
153 known Width Function Instantaneous Unit Hydrograph (WFIUH) was employed
154 (Grimaldi et al., 2012a), viz. the so called WFIUH-1par, which can be calibrated

155 fixing only one parameter related to the value of the concentration time T_c .

156 Finally, flood hydrographs are selected as sequences of positive runoff values
157 separated by at least one zero value. The two main parameters of the COSMO4SUB
158 framework are CN and T_c . The former parameter controls all the processes caus-
159 ing rainfall losses (infiltration, evapotranspiration, etc.), and is mainly related to
160 the soil use and type. Increasing the CN value, a growth of rainfall excess total
161 amount, of rainfall excess maximum intensity, and consequently of durations, are
162 expected. In turn, all the hydrograph attributes are expected to get larger by in-
163 creasing the parameter CN . Instead, T_c corresponds to the IUH base time, so it is
164 representative of the catchment runoff velocity. For large T_c 's, small hydrograph
165 peak discharges and long duration are expected. From a physical perspective, this
166 parameter is mainly related to the soil use, slopes, and drainage network structure.

167 2.3. The simulation procedure

168 The simulation procedure adopted in this work generates long synthetic runoff
169 time series associated with specific parameters responsible of the runoff genera-
170 tion. The plan of action is based on the following steps.

- 171 1. Given a small catchment (described in Section 2.1), the COSMO4SUB
172 model is applied as described in Section 2.2: in turn, runoff time series
173 at 1-hour resolution and 500 years long are generated.

174 2. Varying the parameter CN from 50 to 100 (with a step of $\Delta_{CN} = 5$) and the
175 parameter T_c from 4 to 13 hours (with a step of $\Delta_{T_c} = 1$ hour), a set of 110
176 different combinations were considered: hereinafter, each pair (T_c, CN) will
177 be referred to as a Basin Scenario (BS). Practically, the catchment conditions
178 range from

- 179 • a “minimal” combination (13h and 50), for which it is expected a
180 strong filtering of rainfall input due to the rainfall-runoff transforma-
181 tion, up to
- 182 • a “maximal” combination (4h and 100), representing a fully urbanized
183 catchment.

184 In addition, in order to perform a statistically consistent analysis of the un-
185 certainty inherent the random simulations, a set of $N_S = 100$ independent
186 rainfall time series was generated for each of the 110 BS 's: thus, a total of
187 11,000 data bases are made available.

188 3. Selecting the maximum annual flood Discharge values, and the related hy-
189 drograph Volume and Duration, a set of 500 simulated triples Discharge-
190 Volume-Duration are extracted from each of the 11,000 rainfall-runoff se-
191 ries.

192 Then, several valuable statistical analyses and comparisons can be carried out
193 (see Section 3): viz., the calculation of the degree of association between the
194 relevant variables, the comparison of the Empirical Copulas (*EC*) of interest, the
195 estimate of suitable statistical distances between the dependence structures at play,
196 and the test of equality of the copulas associated with different BS's.

197 It is worth stressing that, at present, only in a few elementary cases, it is possi-
198 ble to guess what should be the features of the copula describing the dependence
199 structure of data generated by a given model: evidently, this is not the case of the
200 involved simulation framework used in this work. Concerning the analysis of the
201 results, the problem can be bypassed by adopting a non-parametric approach, as
202 shown below. Instead, understanding how the model structure/parameterization
203 may affect the copulas of interest is still an open issue, especially when the model
204 is a complex one.

205 **3. Methods and results**

206 As mentioned in the Introduction, the target of this work is to investigate the
207 possible relationships between different Basin Scenarios — as identified by the pa-
208 rameters (T_c , CN) — and the corresponding structures of dependence (viz., the 2-
209 copulas) associated with three relevant pairs of variables: namely, the Discharge-

210 Volume, the Discharge-Duration, and the Volume-Duration.

211 As a fundamental point, all the analyses carried out in the sequel are non-
212 parametric: i.e., no parametric models (univariate and/or multivariate) are fitted to
213 the available data. In particular, all the statistical tests are non-parametric, essen-
214 tially based on the calculation/comparison of the Empirical Copulas of interest.
215 In fact, the use of a parametric approach would involve the selection of specific
216 probability laws, entailing the introduction of unnecessary arbitrariness: in turn,
217 the results would depend upon the choices of the distributions, a nuisance that
218 can be bypassed adopting proper non-parametric techniques, as done in this work.
219 Apparently, the non-parametric tests mentioned below are robust against small
220 sample sizes, e.g. in case short time series were available (say, 20–30 years).

221 It is worth stressing that many of the statistical analyses presented in later
222 Sections assume that the samples investigated consist of independent identically
223 distributed vectors. Here the independence assumption may be taken for granted,
224 both for physical and statistical reasons, since the triples in the series correspond
225 to maximum annual flood events. Instead, changes of the probability distributions
226 at play need to be specifically investigated. For this reason, recent non-parametric
227 univariate/multivariate tests for change point detection (Holmes et al., 2013; Ko-
228 jadinovic, 2014) are used to check whether the series could be considered as “sta-

229 tionary” or not. In particular, for each of the 110 BS’s (T_c, CN)’s examined here,
230 all the pairs Discharge-Volume, Discharge-Duration, and Volume-Duration, and
231 all the triples Discharge-Volume-Duration corresponding the N_S independent se-
232 ries are checked out (for a total of 44,000 tests). The results (not shown for the
233 sake of brevity) suggest that the dependence structures of interest could empir-
234 ically be assumed as “stationary”, since possible change points are statistically
235 detected in less than 5% of the cases (at a standard 5% level), for all the BS’s and
236 variables.

237 3.1. Measuring the degree of association

238 The use of multivariate dependence structures for modeling the variables at
239 play only makes sense when these variables are non-independent. As a first
240 preliminary (but necessary) analysis, two standard measures of association, i.e.
241 the Kendall τ and the Spearman ρ coefficients (Nelsen, 2006; Salvadori et al.,
242 2007), are computed for the three pairs Discharge-Volume, Discharge-Duration,
243 and Volume-Duration, considering all the 110 BS’s of interest here. In particu-
244 lar, for each pair (T_c, CN), both τ and ρ are independently estimated N_S times
245 using all the available series for each BS. The corresponding sample averages and
246 standard deviations are then considered, as well as the p -Values of the related
247 non-parametric Kendall and Spearman tests of independence.

248 Here, only the outcomes concerning the Kendall τ are presented, since the ones
249 involving the Spearman ρ are analogous. Figures 1 show the sample averages of τ ,
250 over all the available N_S series, and the corresponding sample standard deviations,
251 for all the BS's and the three pairs Discharge-Volume, Discharge-Duration, and
252 Volume-Duration.

253 In all cases, the estimated Kendall τ 's are positive, and significantly different
254 from zero, as suggested by the negligible p -Values of the corresponding indepen-
255 dence tests. In turn, all the three pairs of variables are concordant, and cannot
256 be statistically independent. It is worth noting that, apparently, the estimates are
257 well centered around the corresponding mean values, since the variances are very
258 small. A first analysis of the results suggests the following considerations.

- 259 1. The parameter τ changes by varying CN , whereas it is roughly constant by
260 varying T_c .
- 261 2. In general, on the T_c - CN grid, adjacent BS's are associated with close val-
262 ues of τ , viz. the parameter shows a slow variation over the grid.
- 263 3. In the Volume-Duration case, the parameter τ is roughly constant over all
264 the T_c - CN grid, whereas a significant variability is present in the Discharge-
265 Volume and Discharge-Duration cases.
- 266 4. The parameter τ decreases by increasing CN in the Discharge-Volume and

267 Discharge-Duration cases. Physically, for low values of CN , relevant losses
268 are expected, thus reducing the variety of net rainfall events: most of the net
269 hyetographs are concentrated in one-time steps, generating a strong depen-
270 dence between Discharge-Volume (and Discharge-Duration) hydrographs.
271 On the contrary, increasing the CN , the net rainfall events will be similar
272 to the gross rainfall, preserving a complex time structure and, consequently,
273 providing a larger range of volumes (and durations) for a specific Discharge.

274 5. As expected, Discharge-Volume pairs are characterized by the largest values
275 of τ , whereas the lowest degrees of association are present in Discharge-
276 Duration case. The physical reason is due to the rainfall variability and the
277 hyetograph structure. While a flood event with a significant peak will most
278 likely be associated with a large volume (due to the recession curve of the
279 hydrograph), not necessarily a large duration yields a high peak discharge.
280 Indeed, it is possible to have a rainfall event characterized by low intensity
281 values and a long duration that generates a low peak discharge with long
282 duration.

283 6. Concerning the Discharge-Volume case, the results are consistent with those
284 shown in Gaál et al. (2015, Figure 5). Indeed, in the present case study
285 (involving a basin of about 62 Km²), τ approximately ranges in the interval

286 0.6–0.9, essentially the same one reported in the cited paper for a basin of
287 similar dimension.

288 As a conclusion, the results outlined above legitimate the use of a multivariate
289 approach, as done in the present work.

290 3.2. Comparing copulas for different BS's

291 A preliminary rough examination of the similarity/dissimilarity of the depen-
292 dence structures associated with different BS's consists in a visual comparison of
293 the EC's estimated for several combinations of the parameters (T_c, CN)'s. The
294 procedure is as follows.

- 295 1. Three values of T_c are selected (viz., $T_c = 240, 480, 780$ minutes), ranging
296 from the minimum available T_c to the maximum one. Similarly, a triple of
297 CN 's (viz., $CN = 50, 75, 100$) is selected, covering the range of investigated
298 values.
- 299 2. The EC's of the pairs Discharge-Volume, Discharge-Duration, and Volume-
300 Duration are estimated by fixing the value of T_c (or CN), and leaving the
301 other variable vary in the corresponding subset. The EC's are evaluated over
302 a fine 65×65 uniform grid \mathcal{G} on the unit square \mathbf{S} (with a linear resolution
303 equal to $1/64$) — clearly, different grids could be used: thus, a total of

304 $N_G = 65^2 = 4225$ sampling points are considered. Since N_S independent
 305 series are available for each BS, then the grid averages of the EC's (over
 306 the N_S series) are used as reference values: these provide consistent non-
 307 parametric estimates of the copulas of interest at each point of the grid \mathcal{G} .

308 3. The contours (isolines) of the average EC's are then plotted on the same
 309 graphs for selected probability levels: here, 0.1, 0.3, 0.5, 0.7, 0.9. Different
 310 plots can thus be compared.

311 The visual comparison of the (average) EC's estimated for several combina-
 312 tions of the parameters (T_c, CN)'s is shown in Figures 2 and 3, for fixed T_c 's and
 313 CN 's, respectively, and the three pairs Discharge-Volume, Discharge-Duration,
 314 and Volume-Duration. Some preliminary comments are as follows.

315 Apparently, in general the role played by the parameter T_c looks marginal, at
 316 least considering the pairs Discharge-Volume and Discharge-Duration. In fact,
 317 analyzing Figure 2 column-wise, the three plots in the same column are quite
 318 similar, and little changes by increasing the parameter T_c from top to bottom.
 319 Actually, as shown in Figure 3 (where CN is fixed), the isolines corresponding to
 320 the three T_c values are over-lapping, or just barely different.

321 Furthermore, consider the pair Discharge-Volume, and the smallest value of
 322 the parameter CN (viz., 50), as plotted in Figures 2(left column) and 3(top-left).

323 Apparently, the structure of the corresponding (average) 2-copula is similar to the
324 one of the co-monotone 2-copula $\mathbf{M}_2(u, v) = \min(u, v)$ — see, e.g., Salvadori et al.
325 (2007, Figure 3.4) — describing the dependence structure of variables function-
326 ally positively associated. As before (see the comments in Section 3.1 concerning
327 the Kendall τ), for low values of CN almost all of the rainfall amount infiltrates,
328 and the net rainfall is often represented by a single time step, thus yielding a func-
329 tional dependence between Discharge and Volume hydrographs.

330 3.3. Distance and equality of copulas

331 In order to investigate the “closeness” of the dependence structures associated
332 with different Basin Scenarios, we use two suitable statistical tools: namely, a
333 *distance*, and an *equality test*, as discussed below. In the present context, the
334 adjective “statistical” is mandatory, since the copulas of interest are estimated via
335 random samples. A positive distance between copulas is a necessary condition in
336 order to assess the non-equality of copulas. However, it is not a sufficient one: as
337 will be shown later, two copulas may be statistically equal, even if their distance is
338 positive. The explanation of such a counter-intuitive fact is that, here, the notions
339 of distance and equality are taken in a statistical sense, in order to correctly cope
340 with the randomness of the phenomena investigated. Roughly speaking, “small”
341 distances can be (statistically) compatible with the equality of copulas.

342 In order to quantify how distant two dependence structures are (according to a
 343 specific metrics), among many possible choices — see, e.g., Genest et al. (2009)
 344 — the L^2 norm and the L^∞ (or sup) norm criteria are frequently used in applica-
 345 tions. The former one corresponds to a Cramér-von Mises (CvM) approach, while
 346 the latter one is based on statistics of Kolmogorov-Smirnov (KS) type. We antici-
 347 pate here that, for the sake of brevity, only the results concerning the CvM option
 348 will be illustrated in the sequel, the KS case yielding analogous outcomes.

349 Let \mathbf{C}_1 and \mathbf{C}_2 be the copulas associated with, respectively, the BS's $\mathcal{S}_1 =$
 350 $(T_{c,1}, CN_1)$ and $\mathcal{S}_2 = (T_{c,2}, CN_2)$, and denote by $\delta_{1,2}$ the distance between \mathbf{C}_1 and
 351 \mathbf{C}_2 . Then, in the CvM case, the following definition may be introduced:

$$\delta_{1,2} = 24 \cdot \int_{\mathbf{u} \in \mathcal{S}} (\mathbf{C}_1(\mathbf{u}) - \mathbf{C}_2(\mathbf{u}))^2 d\mathbf{u}, \quad (1)$$

352 where the coefficient “24” is used to normalize δ into the interval $[0, 1]$ (Nelsen,
 353 2006). A consistent empirical estimator $\tilde{\delta}_{1,2}$ of $\delta_{1,2}$ is given by

$$\tilde{\delta}_{1,2} = 24 \cdot \sum_{\mathbf{u} \in \mathcal{G}} (\tilde{\mathbf{C}}_1(\mathbf{u}) - \tilde{\mathbf{C}}_2(\mathbf{u}))^2 / N_G, \quad (2)$$

354 where $\tilde{\mathbf{C}}_\bullet$ denotes the EC corresponding to the copula \mathbf{C}_\bullet . For the sake of com-
 355 pleteness, the KS case is outlined in Appendix B.

356 Practically, the estimation of $\delta_{1,2}$ is carried out as follows.

357 1. Two different BS's \mathcal{S}_1 and \mathcal{S}_2 are selected, and the EC's of the three pairs

358 Discharge-Volume, Discharge-Duration, and Volume-Duration are estimated
359 for both \mathcal{S}_1 and \mathcal{S}_2 over the grid \mathcal{G} .

360 2. Since N_s independent series are available for each BS, then N_s values of $\tilde{\delta}_{1,2}$
361 can be calculated, using either Eq. (2) or Eq. (B.2): clearly, such a sample
362 can be used to extract valuable empirical information about the distribution
363 of $\delta_{1,2}$. For instance, the sample mean (or median) may provide indications
364 about the central value of the distance, the sample standard deviation may
365 supply information about its variability, and suitable non-parametric empir-
366 ical Confidence Intervals may be calculated as well, by properly using the
367 associated Order Statistics.

368 It is worth pointing out that, per se, the notion of distance has only a relative mean-
369 ing, and should only be used for doing relative comparisons: roughly speaking,
370 larger distances in general correspond to more dissimilar copulas.

371 In fact, the crucial question whether two copulas are equal or not can only
372 be answered via a suitable statistical test. A non-parametric procedure for testing
373 the equality between copulas has recently been outlined in Rémillard and Scaillet
374 (2009); Rémillard and Plante (2012): the test indicates whether the null hypothe-
375 sis \mathcal{H}_0 that two copulas are the same should be statistically rejected or not. In the
376 present context, the test is used to compare copulas associated with pairs of vari-

377 ables simulated under different Basin Scenarios: clearly, if \mathcal{H}_0 cannot be statis-
 378 tically rejected, then presumably the data share a common dependence structure.
 379 Here, given two different BS's \mathcal{S}_1 and \mathcal{S}_2 , the N_S copulas of the pairs Discharge-
 380 Volume, Discharge-Duration, and Volume-Duration are compared at a standard
 381 5% level. In turn, the percentage of non-rejection of \mathcal{H}_0 (over the N_S tests) may
 382 provide valuable empirical information about the similarity of the copulas of inter-
 383 est: clearly, large percentages of non-rejection may support the statistical equality
 384 of the copulas considered.

385 3.4. Investigating the compatibility of BS's

386 As mentioned above, a Basin Scenario is characterized via the corresponding
 387 pair (T_c, CN) , i.e. a two-parameter identification: Figure 4(top-left) illustrates the
 388 T_c - CN grid considered here, consisting of 110 different BS's. Note that, one step
 389 in the CN direction corresponds to a jump of size $\Delta_{CN} = 5$, while one step in the
 390 T_c direction corresponds to a jump of size $\Delta_{T_c} = 60$ minutes. For instance, the
 391 two BS's X and Y plotted in the picture are 7 units far apart in the CN direction,
 392 and 5 units far apart in the T_c direction: evidently, X and Y correspond to (very)
 393 dissimilar hydrological scenarios.

394 In this work, we are interested in understanding whether, and how, the param-
 395 eters T_c and CN may affect the dependence structures of the data bases simulated

396 according to different (T_c, CN) settings. The methodology outlined in this Section
397 represents a preliminary necessary step in order to assess under which conditions
398 *data sets generated according to different BS paradigms could be interchanged*
399 (viz., are “compatible”), when the interest is in the dependence structures at play.

400 In order to investigate the compatibility of different Basin Scenarios, we intro-
401 duce here several concepts, exploiting the statistical tools outlined in the previous
402 Section. In particular, general definitions of BS compatibility are proposed, based
403 either on an “individual” or an “ensemble” rationale involving the dependence
404 structures of interest. In the former case, the copula corresponding to a reference
405 BS X is compared with the one associated with a single neighbor BS Y (viz., only
406 two BS’s at a time are involved). In the latter case, the copula corresponding to
407 a reference BS X is compared with a family of copulas associated with a set of
408 neighboring BS’s Y’s (viz., a number of different BS’s is considered at a time).

409 Here, three different approaches are outlined: all the notions are based on the
410 test of equality mentioned in Section 3.3, as well as on the distance δ . Actually,
411 three further strategies can be considered: these are briefly introduced in Appendix
412 A, and illustrated in Figure 4; however, for the sake of brevity, they will not be
413 discussed in this work.

414 **Pairwise.** Consider two generic BS’s X and Y, such as those illustrated in Figure

415 4(top-left). Their Pairwise BS (individual) distance is defined as the average
 416 δ distance (over the N_s series) between them. Correspondingly, the Pairwise
 417 BS (individual) compatibility between X and Y is defined as the percentage
 418 of equality tests (over the N_s series) for which the assumption \mathcal{H}_0 should
 419 not be rejected.

420 **Foreword-CN (fCN).** The fCN BS (individual) distance of size d is defined as
 421 the average δ distance (over the N_s series) between the BS's $\mathcal{S}_1 = (T_c, CN)$
 422 and $\mathcal{S}_2 = (T_c, CN + d \cdot \Delta_{CN})$. The notion is illustrated in Figure 4(bottom-
 423 right), using d 's ranging from 1 to 4. Correspondingly, their fCN BS (indi-
 424 vidual) compatibility is defined as the percentage of equality tests (over the
 425 N_s series) for which the assumption \mathcal{H}_0 should not be rejected. Clearly, the
 426 fCN approach is a special case of the Pairwise one: here, the parameter T_c
 427 is kept fixed.

428 **Ring.** The Ring BS (ensemble) distance of size d is defined as the average (over
 429 the N_s series) of the average δ distance between the BS $\mathcal{S} = (T_c, CN)$ and all
 430 the neighboring BS's lying on a suitable " d -ring" surrounding \mathcal{S} on the T_c -
 431 CN grid: viz., at least one of the neighbor parameters T_c or CN is equal to
 432 $T_c \pm d \cdot \Delta_{T_c}$ or $CN \pm d \cdot \Delta_{CN}$. The notion is illustrated in Figure 4(top-right), for

433 d equal to 1 and 2. Practically, the Ring distance is the average of all the BS
 434 distances considering the neighbors of \mathcal{S} positioned on a suitable frame sur-
 435 rounding \mathcal{S} on the T_c - CN grid. Note that the Ring approach averages over
 436 “full” changes of the basin parameters, and may provide an overall picture
 437 of the differences between the BS \mathcal{S} and its neighbors. Correspondingly, the
 438 Ring BS (ensemble) compatibility is defined as the average percentage of
 439 equality tests (over the N_s series) for which the assumption \mathcal{H}_0 should not
 440 be rejected, when testing the equality between \mathcal{S} and all its Ring neighbors.

441 Clearly, BS distances and compatibilities can be computed by considering
 442 only the neighboring BS’s inside the T_c - CN grid: in fact, for large d ’s, some of the
 443 surrounding BS’s may not lie inside the T_c - CN grid, and would not be admissi-
 444 ble. From a statistical point of view, this means that some averages are calculated
 445 using different sample sizes, but such a nuisance is geometrically unavoidable.

446 As an illustration of the Pairwise approach, consider the three BS’s labelled
 447 X, Y, and Z in Figure 4(top-left). The sample averages and standard deviations
 448 (over the N_s series) of the distance δ between the pairs $\{X,Y\}$, $\{X,Z\}$, and $\{Y,Z\}$
 449 are reported in Table 2, considering the variables Discharge-Volume, Discharge-
 450 Duration, and Volume-Duration. It is interesting to note that, in general, the values
 451 of δ for the pair $\{X,Y\}$ are larger than the corresponding ones for the pairs $\{X,Z\}$

452 and $\{Y,Z\}$.

453 The results concerning the fCN and the Ring strategies are illustrated in Fig-
454 ures 5–6 for $d = 1$, and in Figures 7–8 for $d = 2$, and are commented below.

455 **The case $d = 1$.** Both the fCN and the Ring average distances look rather small
456 and homogenous over all the T_c - CN grid, and the very small standard de-
457 viations indicate the presence of negligible fluctuations, for all the BS's
458 considered.

459 **The case $d = 2$.** As a difference with the previous case, both the fCN and the
460 Ring average distances get larger, and also more heterogeneously distributed
461 over the T_c - CN grid. In general, also the standard deviations slightly in-
462 crease, indicating a greater variability.

463 The results presented above are not surprising: in fact, larger d 's are expected to
464 yield more distant BS's under the fCN and Ring approaches.

465 3.5. *Compatibility Maps*

466 As mentioned above, the percentage of non-rejection of \mathcal{H}_0 may provide an
467 empirical tool to evaluate the similarity of the copulas corresponding to different
468 BS's. A comparison procedure can be put at work by introducing the so-called

469 Compatibility Maps (*CM*). In the present context, the resolution of the maps is
 470 $\Delta_{CN} = 5$ in the *CN* direction, and $\Delta_{T_c} = 60$ minutes in the T_c direction.

471 The procedure is as follows: for the sake of brevity, only the Pairwise case is
 472 considered here, the others being analogous. Firstly, a BS \mathcal{S} is selected, and the
 473 Pairwise compatibility of \mathcal{S} with respect to all the remaining BS's is computed: in
 474 the present setting, there are 109 possible neighbors. Secondly, the plot on the T_c -
 475 *CN* grid of the non-rejection percentages of \mathcal{H}_0 provides a Pairwise *CM* for the
 476 BS \mathcal{S} . In turn, the data collected under BS's whose percentage of non-rejection is
 477 large enough (say, larger than 90%, or 95%, or any other suitable threshold) could
 478 be labelled as “compatible” with the observations in \mathcal{S} .

479 As an illustration, here the three BS's labelled X, Y, and Z in Figure 4(top-
 480 left) are assumed to characterize the basins of reference: the target is to spot
 481 which are the BS's “compatible” with the reference ones, by constructing the
 482 corresponding *CM*'s, and identifying possibly admissible neighboring BS's. The
 483 results are shown in Figure 9. The situation varies by considering different pairs
 484 of variables (viz., either the Discharge-Volume, or the Discharge-Duration, or the
 485 Volume-Duration ones), and the (T_c, CN) parameters associated with X, Y, or Z.
 486 Considering the Volume-Duration case, it is clear that at least all the BS's in a
 487 surrounding ring of size $d = 1$ would be admissible, being the percentages of non-

488 rejection close to 100%. Instead, in the Discharge-Duration case, only some of the
489 BS's in a surrounding ring of size $d = 1$ would be fully admissible, although the
490 percentages of non-rejection are large for all the eight neighbors of the reference
491 BS's. Finally, in the Discharge-Volume case, the BS X turns out to be compatible
492 with a surrounding ring of BS's of size $d = 2$ (and similarly for Z, considering a
493 ring of size $d = 1$), whereas the BS Y seems to be compatible only with the closest
494 BS's in the same column of the T_c -CN grid. Further examples of Compatibility
495 Maps are shown in Figures 5–8, considering both the Foreword-CN and the Ring
496 approaches, and grid distances of size $d = 1$ and $d = 2$.

497 3.6. Merging different data bases

498 As already mentioned above, often the size of a sample collected at a given
499 site may be insufficient for statistical purposes: for instance, the estimate and
500 validation of multivariate dependence structures may not be accurate by using in-
501 adequate data bases. In turn, the idea is to try and increase the actual sample size
502 by “merging” the available data with other observations collected in “compati-
503 ble” basins. For this purpose, the Compatibility Maps introduced in Section 3.5
504 could be used to understand and single out which external data bases might be
505 appropriate to flesh out the scarce one.

506 As for the concepts of basin compatibility, also the merging task may be either

507 individual or ensemble: viz., the external data may be picked up from a single
508 data base, or from a number of data bases. In the former case, the Pairwise (or
509 fT_c , or fCN) procedure provides a natural selection criterion. In the latter case, a
510 global survey can be carried out simultaneously over all the external data bases by
511 using an ensemble criterion (e.g., the Ring, the Diagonal, or the Cross strategy):
512 for instance, considering the Ring approach, the BS's of the data bases involved
513 should belong to an admissible ring on the (T_c, CN) -grid, centered on the target
514 BS S .

515 A workable methodology for merging distinct data bases (possibly of differ-
516 ent sizes) is as follows: the pseudo-observations (i.e., the ranks divided by the
517 sample size — see, e.g., Genest and Favre (2007)) associated with the available
518 data are first calculated, and then combined. In case ties (i.e., repeated values)
519 were present, then suitable randomizations may be performed in order to remove
520 them (Kojadinovic and Yan, 2010; Hofert et al., 2015). Note that all the stan-
521 dard non-parametric procedures for estimating copulas are based on the pseudo-
522 observations.

523 A simple method to test whether the merging procedure outlined above makes
524 sense is as follows. First, a series is split into two halves, and the pseudo-obs-
525 ervations of each subset are computed, and subsequently combined. Then, the

526 EC associated with the merged data base is compared with the original one via
527 an equality test, in order to statistically check whether the two are “compatible”:
528 clearly, this represents a necessary condition for the validity of the merging op-
529 eration. In the present study, such a test was carried out considering all the 110
530 BS’s of the T_c -CN grid, and all the N_S series: as a result, only in a dozen out
531 of the 33,000 equality tests the statistical hypothesis that the original and merged
532 copulas were the same was rejected (at a 5% level).

533 A study of the performances of the merging techniques outlined above was
534 carried out by considering the three BS’s X, Y, and Z already investigated in Sec-
535 tion 3.4 — see Figure 4(top-left). In particular, the Ring merging strategy was
536 adopted, using $d = 1$ and $d = 2$. Tables 3 and 4 show, respectively, the cor-
537 responding percentages of non-rejection of \mathcal{H}_0 : here the EC’s of the simulated
538 series are compared with those of the merged data bases of the admissible Ring
539 neighbors. In addition, for the sake of visual comparison, Figures 10 and 11 show,
540 respectively, the comparison between the EC’s of the “true” and the “merged” se-
541 ries (averaged over the N_S series), for $d = 1$ and $d = 2$, considering the BS’s
542 X, Y, and Z, and the pairs Discharge-Volume, Discharge-Duration, and Volume-
543 Duration.

544 As expected, in general the percentages of non-rejection of \mathcal{H}_0 decrease by

545 increasing the grid distance d : obviously, further neighbors ($d = 2$) are less com-
546 patible than closer ones ($d = 1$). It is interesting to note that, in the case $d = 1$, all
547 the percentages are larger than 90%: this may indicate a significant compatibility
548 between the series of the BS's X, Y, and Z, and those of the corresponding Ring
549 neighbors. The analysis is visually confirmed by the plots shown in Figures 10
550 and 11: in fact, large percentages in Tables 3 and 4 are generally associated with
551 almost overlapping isolines, whereas significant differences of the contour plots
552 generally correspond to small percentages.

553 **4. Conclusions**

554 This paper investigates a specific, yet unexplored, aspect of hydrologic simi-
555 larity, viz. the catchment compatibility with respect to the similarity of the cop-
556 ulas at play under different Basin Scenarios. In particular the possible relations
557 between hydrological responses and corresponding dependence structures are an-
558 alyzed by means of simulation tests, where relevant hydrological variables are
559 generated in a small catchment under controlled conditions. For this purpose, a
560 hydrological model is used, keeping constant both the climatic forcing (synthetic
561 rainfall time series) and the topography, whereas the Curve Number and the Con-
562 centration Time are varied. Such a procedure provides a number of scenarios

563 (110, in the present study) that differ from one another only for the parameter
564 responsible of the infiltration and losses processes, and the runoff velocity in the
565 watershed. For each scenario, 500 years of runoff are simulated, and the triples
566 of Discharge-Volume-Duration corresponding to maximum annual floods are se-
567 lected.

568 Measures of association, empirical copula visual comparison, Cramér-von
569 Mises distances between dependence structures, non-parametric procedures for
570 testing the equality among bivariate copulas, and a variety of compatibility defi-
571 nitions (individual and/or ensemble) are applied on the pairs Discharge-Volume,
572 Discharge-Duration, and Volume-Duration, yielding suitable Compatibility Maps.
573 This latter tool provides an easy way to (statistically) evaluate which T_c - CN com-
574 binations may yield compatible dependence structures.

575 On the one hand, these preliminary results could pave the way for a possible
576 new approach concerning the identification of “compatible” basins. On the other
577 hand, given two (or more) “compatible” catchments, workable criteria and pro-
578 cedures have been stated for merging the available data: clearly, increasing the
579 sample size may improve the estimation of the copulas at play, providing an use-
580 ful tool for bypassing the lack of hydrological observation (a crucial problem in
581 applications).

582 The results presented in this work suggest further investigations both on the
583 relation between dependence structure and hydrological parameters, and on the
584 practical implications of the approach proposed. Indeed, the next research step
585 will involve a more complex model, as well as a wider range of climatic forc-
586 ings, in order to achieve a thorough survey of the relations between dependence
587 structure and physical attributes. In addition, a set of case studies (with limited
588 observations) will be tested, in order to check the practical potentiality of the
589 merging procedure and the compatibility maps.

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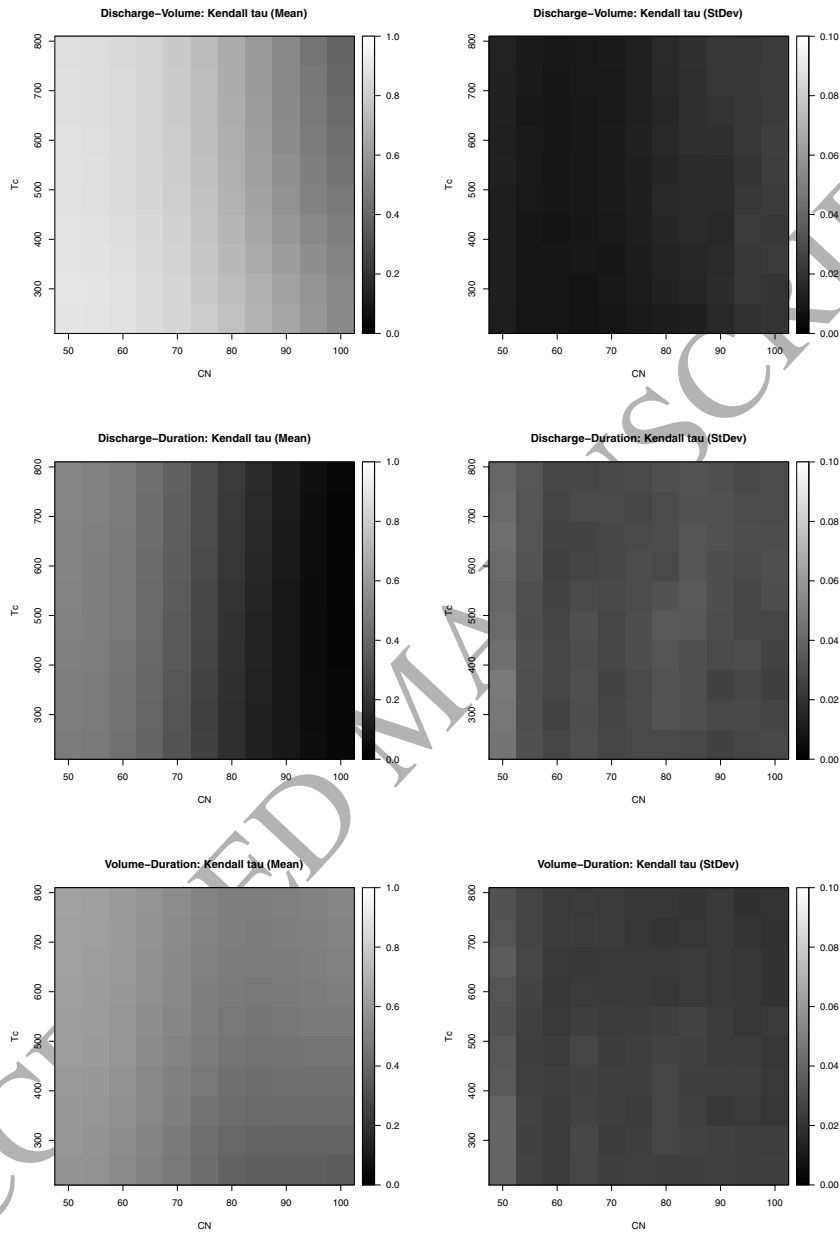


Figure 1: Estimates of the Kendall τ , over all the available N_S series, for different BS's and variables — see text: (*left column*) sample average; (*right column*) sample standard deviation.

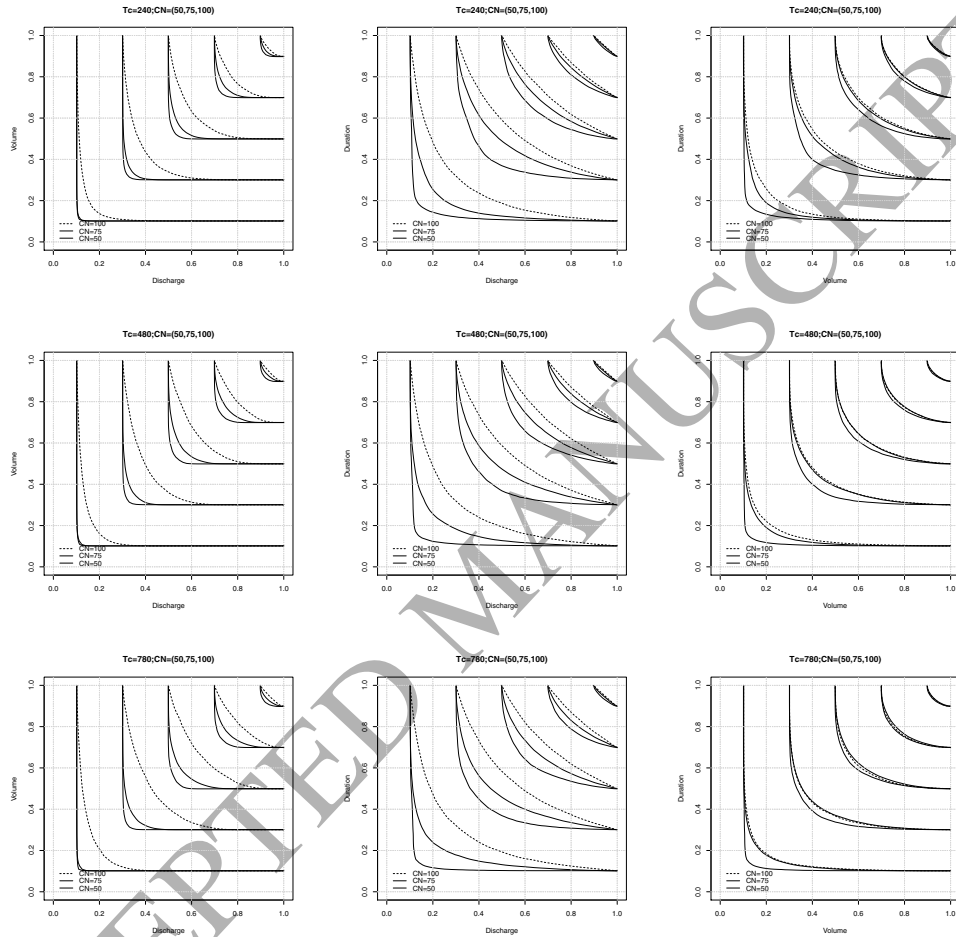


Figure 2: Comparison of the (average) Empirical Copulas, for fixed T_c 's (rows) and different CN 's — see text: (left column) the Discharge-Volume pair, (middle column) the Discharge-Duration pair, and (right column) the Volume-Duration pair.

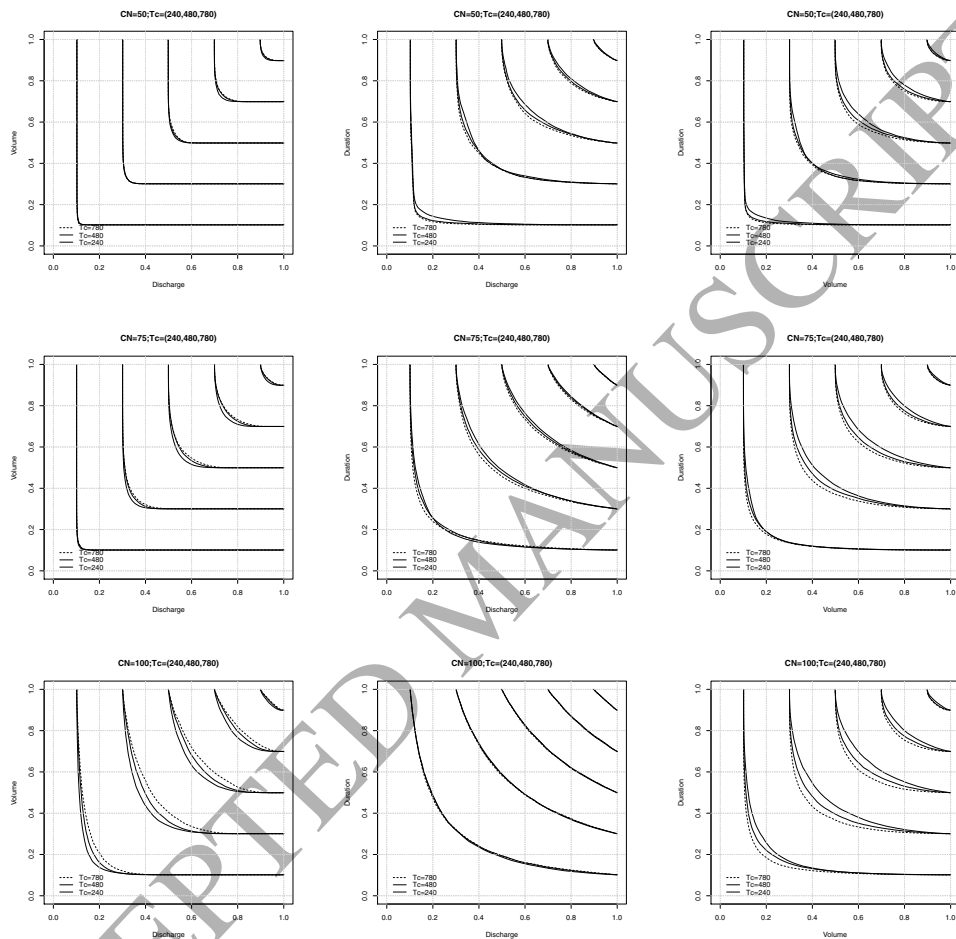


Figure 3: Comparison of the (average) Empirical Copulas, for fixed CN 's (*rows*) and different T_c 's — see text: (*left column*) the Discharge-Volume pair, (*middle column*) the Discharge-Duration pair, and (*right column*) the Volume-Duration pair.

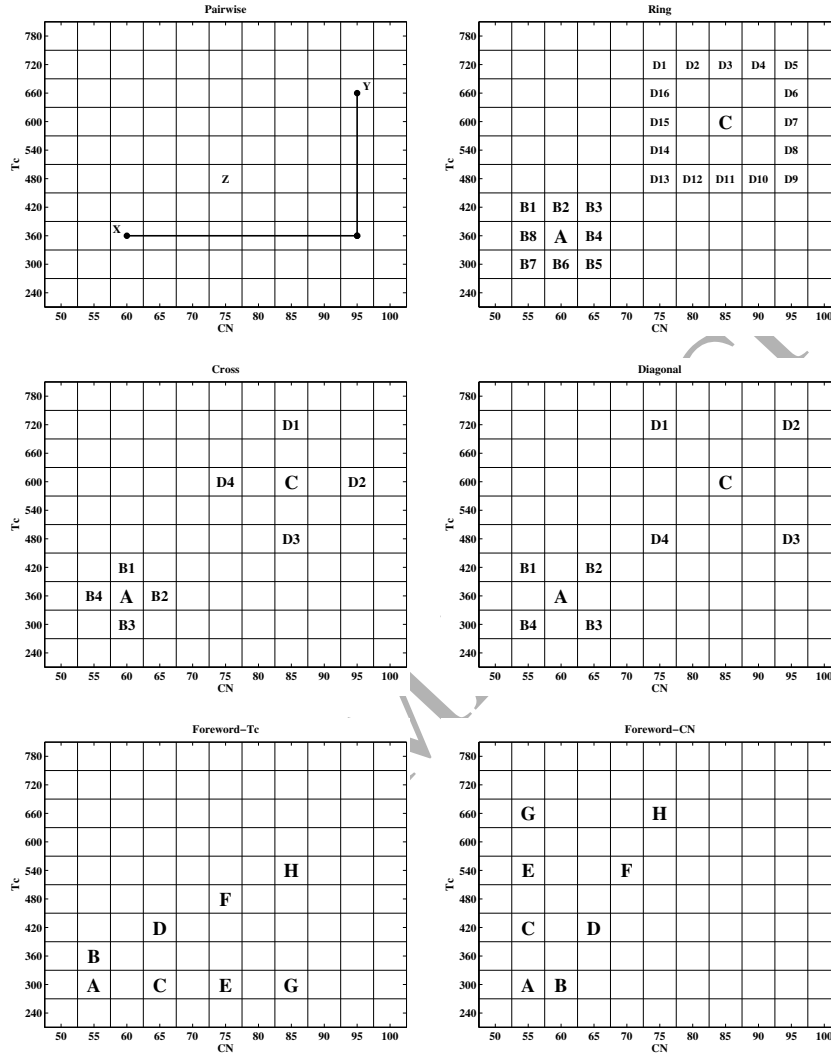


Figure 4: Illustration of the strategies for investigating the compatibility of Basin Scenario outlined in Section 3.4. (Top-left) Pairwise. (Top-right) Ring. (Middle-left) Cross. (Middle-right) Diagonal. (Bottom-left) Foreword- T_c . (Bottom-right) Foreword-CN. Four different d 's are shown: respectively, $d = 1$ (A and B's pairs), $d = 2$ (C and D's pairs), $d = 3$ (E and F's pairs), and $d = 4$ (G and H's pairs). In the Top-left panel is also indicated the "central" BS Z considered in Section 3.4.

3.4.

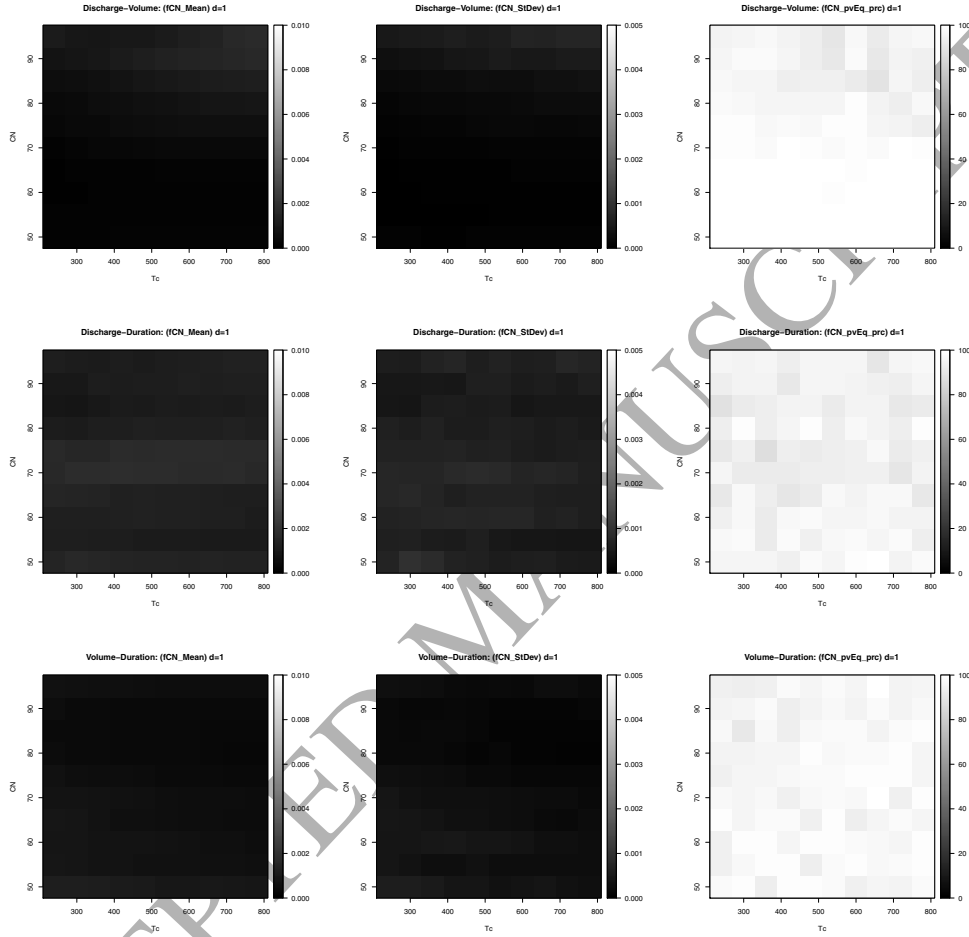


Figure 5: Values of the Foreword- CN distances, over all the available N_S series, for different BS's and variables, using $d = 1$ — see text: (*left column*) sample average; (*middle column*) sample standard deviation. The *right column* shows the corresponding CM's: indicated are the percentages of non-rejection of \mathcal{H}_0 .

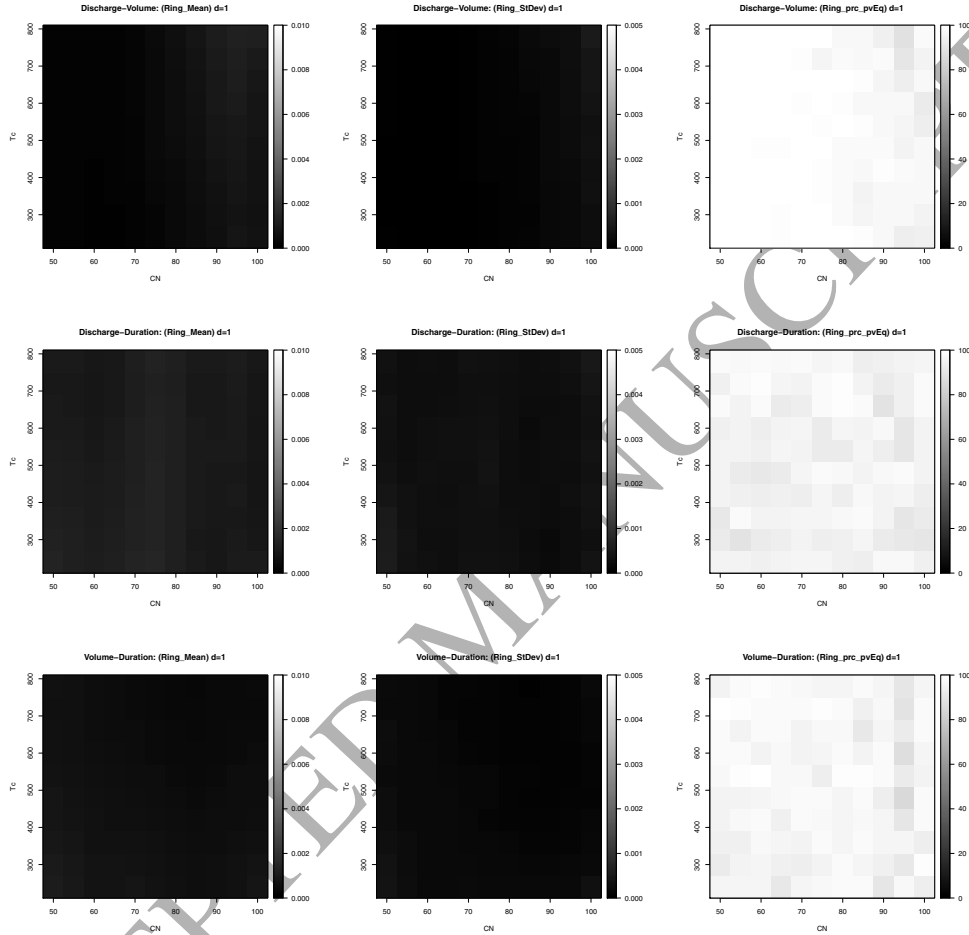


Figure 6: Values of the Ring distances, over all the available N_S series, for different BS's and variables, using $d = 1$ — see text: (*left* column) sample average; (*middle* column) sample standard deviation. The *right* column shows the corresponding CM's: indicated are the percentages of non-rejection of \mathcal{H}_0 .

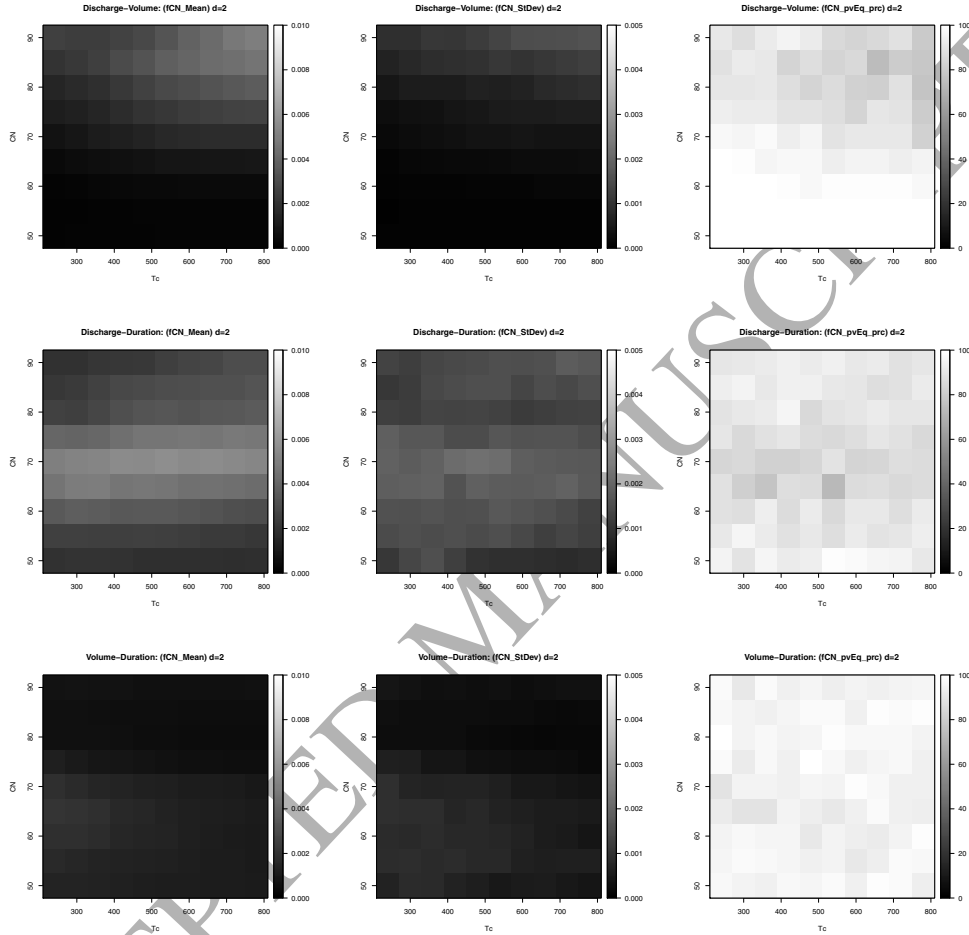


Figure 7: Values of the Foreword- CN distances, over all the available N_S series, for different BS's and variables, using $d = 2$ — see text: (*left* column) sample average; (*middle* column) sample standard deviation. The *right* column shows the corresponding CM's: indicated are the percentages of non-rejection of \mathcal{H}_0 .

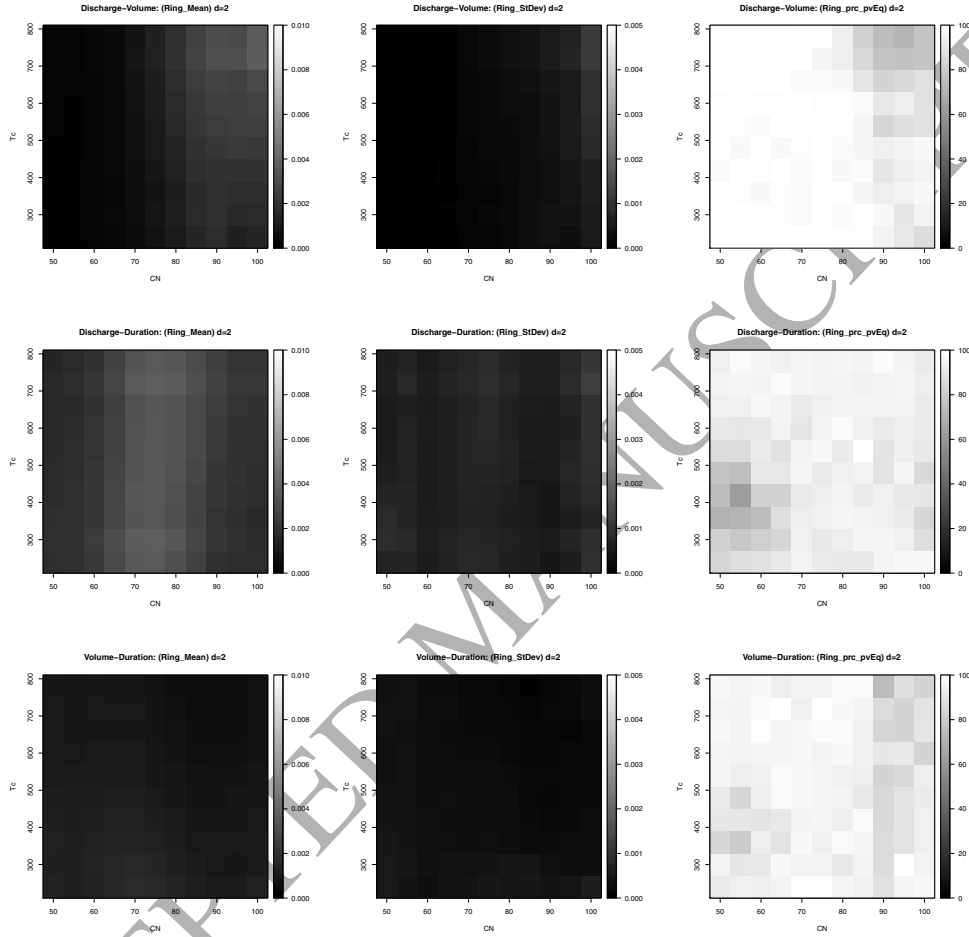


Figure 8: Values of the Ring distances, over all the available N_S series, for different BS's and variables, using $d = 2$ — see text: (*left* column) sample average; (*middle* column) sample standard deviation. The *right* column shows the corresponding CM's: indicated are the percentages of non-rejection of \mathcal{H}_0 .

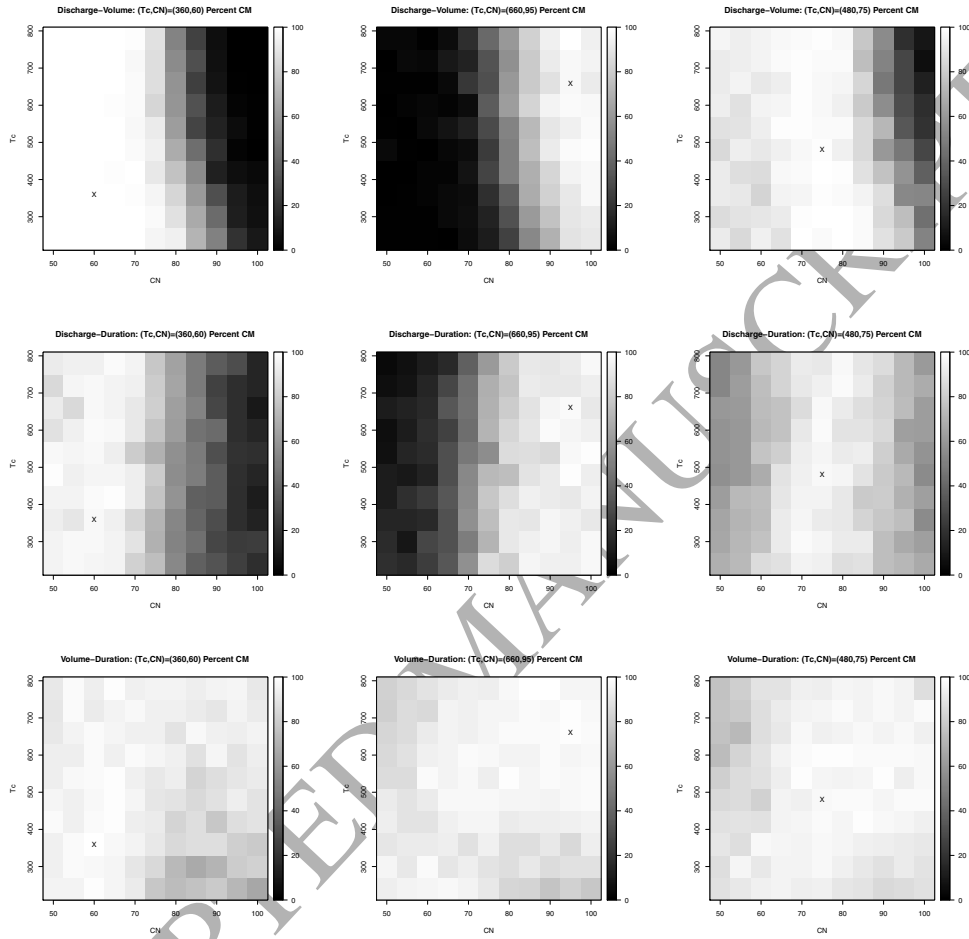


Figure 9: Pairwise Compatibility Maps, for the three BS's labelled X, Y, and Z in Figure 4 (top-left), for different variables — see text: the reference BS is marked as “x”. (*Left column*) CM's for the BS X ($T_c = 360, CN = 60$). (*Middle column*) CM's for the BS Y ($T_c = 660, CN = 95$). (*Right column*) CM's for the BS Z ($T_c = 480, CN = 75$). The colormap indicates the percentages of non-rejection of H_0 .

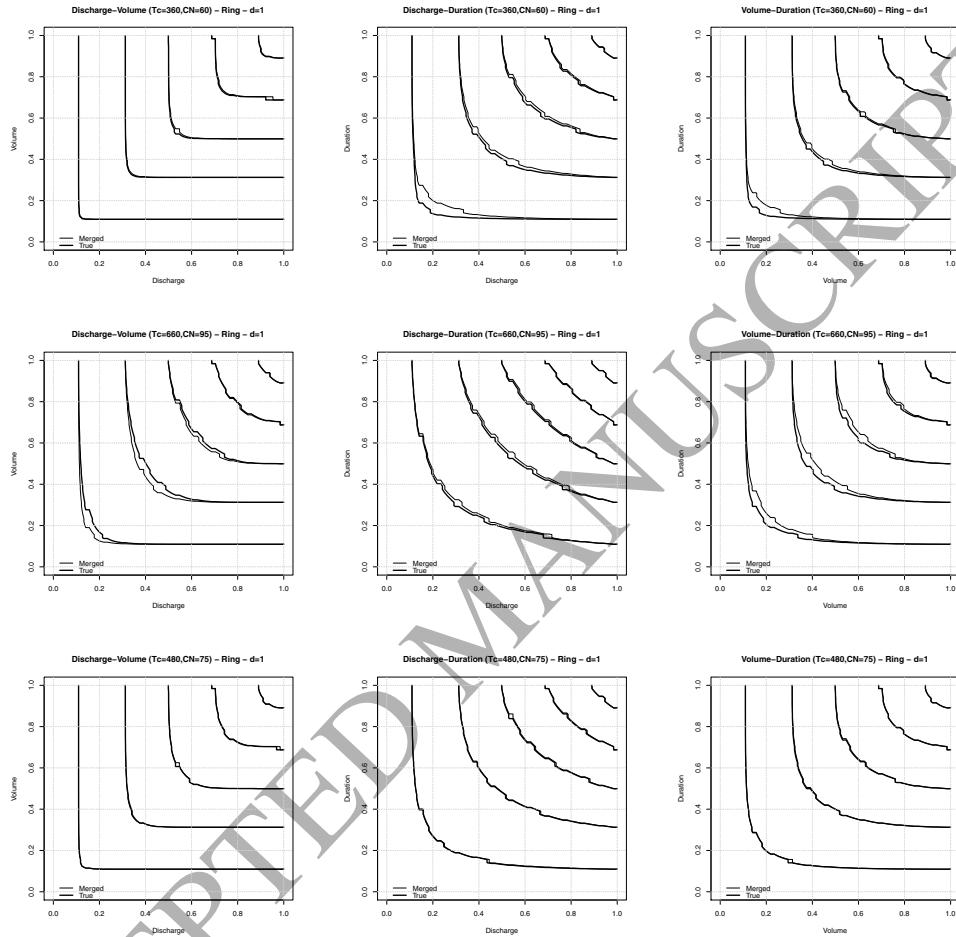


Figure 10: Comparison between the (average) Empirical Copulas of the “true” and the “merged” series, for $d = 1$, considering the BS’s X (top row), Y (middle row), and Z (bottom row) — see text: (left column) the Discharge-Volume pair, (middle column) the Discharge-Duration pair, and (right column) the Volume-Duration pair.

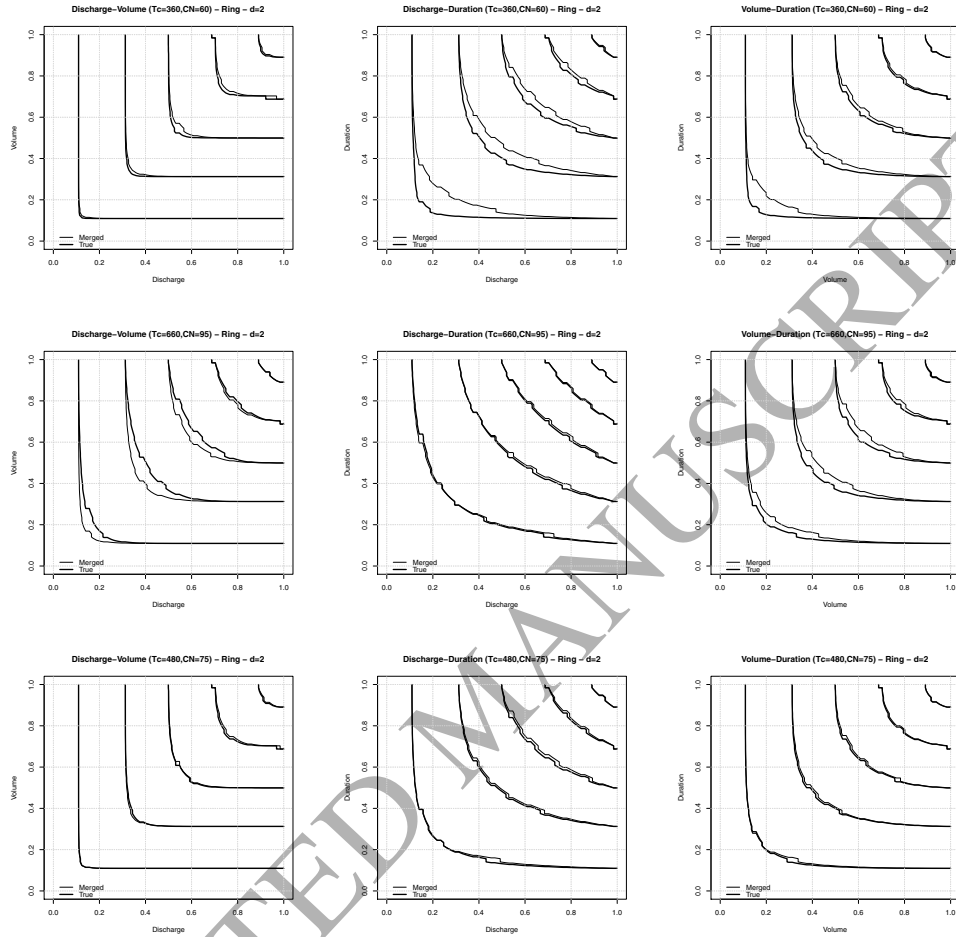
Figure 11: Same as Figure 10, for $d = 2$.

Table 1: The main features of the Torbido watershed case study — see text.

DEM source	IGMI
Cell size (m)	20
Precision	integer
Outlet coordinate North - UTM 33 N	4721505
Outlet coordinate East - UTM 33 N	272623
Area (Km ²)	61.67
Min elevation (m)	85.0
Max elevation (m)	625.0
Mean elevation (m)	373.8
Max slope (%)	375.0
Mean slope (%)	21.9
Main channel length (Km)	24.50
Maximum divide-outlet distance (Km)	25.81

Table 2: Sample averages and standard deviations (in parentheses) of the distance δ , for the pairs of BS's {X,Y}, {X,Z}, and {Y,Z}, considering the variables Discharge-Volume, Discharge-Duration, and Volume-Duration — see text.

Pair	Dis.-Vol.	Dis.-Dur.	Vol.-Dur.
(X,Y)	0.0204 (0.0030)	0.0332 (0.0067)	0.0023 (0.0009)
(X,Z)	0.0013 (0.0004)	0.0075 (0.0031)	0.0023 (0.0012)
(Y,Z)	0.0129 (0.0022)	0.0113 (0.0035)	0.0010 (0.0005)

Table 3: Percentages of non-rejection of \mathcal{H}_0 under the Ring strategy, for $d = 1$, considering the pairs Discharge-Volume, Discharge-Duration, and Volume-Duration — see text.

BS	(T_c, CN)	Dis.-Vol.	Dis.-Dur.	Vol.-Dur.
X	(360, 60)	99.93	94.17	95.97
Y	(660, 95)	92.70	94.10	91.80
Z	(480, 75)	99.33	95.73	96.47

Table 4: Same as Table 3, for $d = 2$.

BS	(T_c, CN)	Dis.-Vol.	Dis.-Dur.	Vol.-Dur.
X	(360, 60)	98.63	74.03	87.10
Y	(660, 95)	83.47	94.13	87.43
Z	(480, 75)	99.03	94.87	95.33

794 **Appendix A. More compatibility criteria**

795 In addition to the compatibility criteria illustrated in Section 3.4, the following
796 three strategies can also be considered.

797 1. The Foreword- T_c BS (individual) distance of size d is defined as the average
798 δ distance (over the N_s series) between the BS's $\mathcal{S}_1 = (T_c, CN)$ and $\mathcal{S}_2 =$
799 $(T_c + d \cdot \Delta_{T_c}, CN)$. The notion is illustrated in Figure 4(bottom-left), using
800 d 's ranging from 1 to 4. A corresponding Foreword- T_c BS compatibility
801 can be introduced.

802 2. The Cross BS (ensemble) distance of size d is defined as the average (over
803 the N_s series) of the average δ distance between the BS $\mathcal{S} = (T_c, CN)$ and
804 the neighboring BS's $(T_c \pm d \cdot \Delta_{T_c}, CN)$ and $(T_c, CN \pm d \cdot \Delta_{CN})$. The notion
805 is illustrated in Figure 4(middle-left), for d equal to 1 and 2. Practically,
806 the Cross distance is the average of the BS distances considering only the
807 neighbors of \mathcal{S} positioned on a suitable “cross” on the T_c - CN grid. Note
808 that, each of the neighbor BS's have only one of the parameters CN or T_c
809 different from the reference BS \mathcal{S} : this corresponds to average over “partial”
810 changes of the basin parameters. A corresponding Cross BS compatibility
811 can be introduced.

812 3. The Diagonal BS (ensemble) distance of size d is defined as the average

813 (over the N_s series) of the average δ distance between the BS $\mathcal{S} = (T_c, CN)$
 814 and the neighboring BS's $(T_c \pm d \cdot \Delta_{T_c}, CN \pm d \cdot \Delta_{CN})$. The notion is illustrated
 815 in Figure 4(middle-right), for d equal to 1 and 2. Practically, the Diagonal
 816 distance is the average of the BS distances considering only the neighbors
 817 of \mathcal{S} positioned on a suitable “crux decussata” (Saint Andrew’s Cross) on
 818 the T_c – CN grid. Note that, each of the neighbor BS’s have both of the
 819 parameters CN and T_c different from the reference BS \mathcal{S} : this corresponds
 820 to average over “total” changes of the basin parameters. A corresponding
 821 Diagonal BS compatibility can be introduced.

822 Practically, the Foreword- T_c approach is a special case of the Pairwise one, while
 823 the Cross and Diagonal strategies are sub-cases of the Ring one.

824 **Appendix B. KS distance of copulas**

825 Let \mathbf{C}_1 and \mathbf{C}_2 be the copulas associated with, respectively, the BS’s $\mathcal{S}_1 =$
 826 $(T_{c,1}, CN_1)$ and $\mathcal{S}_2 = (T_{c,2}, CN_2)$, and denote by $\delta_{1,2}$ the distance between \mathbf{C}_1 and
 827 \mathbf{C}_2 . Then, adopting a Kolmogorov-Smirnov (KS) approach, the following defini-
 828 tion may be introduced:

$$\delta_{1,2} = 2 \cdot \sup_{\mathbf{u} \in \mathbf{S}} |\mathbf{C}_1(\mathbf{u}) - \mathbf{C}_2(\mathbf{u})|, \quad (\text{B.1})$$

829 where the coefficient “2” is used to normalize δ into the interval $[0, 1]$ (Nelsen,
830 2006). A consistent empirical estimator $\tilde{\delta}_{1,2}$ of $\delta_{1,2}$ is given by

$$\tilde{\delta}_{1,2} = 2 \cdot \max_{\mathbf{u} \in \mathcal{G}} |\tilde{\mathbf{C}}_1(\mathbf{u}) - \tilde{\mathbf{C}}_2(\mathbf{u})|. \quad (\text{B.2})$$

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